Hydraulic and Coastal Engineering Design





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Goal of this course

- •Role of the practicing hydraulic/coastal engineer.
- •Apply theory to design problems.
- •Apply models used in the industry.

Instructor

- •Jeremy Bricker, P.E., Ph.D.
- Education
 - BS Mechanical Engineering, Rutgers University (New Jersey)
 MS, PhD Civil Engineering, Stanford University (California)
- Licensing
 - •PE (Civil) in California and Washington
- •Experience
 - •URS Corporation (Engineering and Design) staff Hydraulic and Coastal Engineer

Project Experience

- •Hydrology San Clemente Dam Removal
- •River hydraulics Delta Habitat and Conveyance Project
- •Coastal hydraulics San Francisco Airport runway extension sediment transport study
- •Wave mechanics San Francisco Public Utility Commission wave power feasibility and erosion study

Course Outline

- •Hydrology
- •River hydraulics
- •Coastal hydraulics
- •Wave mechanics

Hydrology

- •Dam safety!
- •Design spillways to pass the maximum possible flood!
- •Gauged basins extreme value analysis
- •Ungauged basins Rational Method
- •Weir and orifice discharge equations

Source: USGS









Hydrology – gauged basins Sacramento River Annual Maxima and Exceedance Curve



Hydrology – gauged basins Sacramento River Annual Maxima and Exceedence Curve



Hydrology – ungauged basins: Probable Maximum Precipitation, NOAA HMR 59



•Design storm

•Find flow rate at location of structure



Basin length, slope, and time of concentrationOverland flow, concentrated flow, channel flow



•Land cover and soil type



•Land cover and soil type



•Find flow at structure



•Dams, gates, and spillways – San Clemente Dam, California











San Clemente dam discharge rating curve



•Dams, gates, and spillways – San Clemente Dam, California



•Spillways – Los Vaqueros Dam, California



- •Overtopping Fujinuma Dam, Fukushima
- •Slumped due to quake, then overtopped and scoured
- •Same could happen for inadequate spillway



Spillways – Hoover Dam, Nevada/Arizona
Glen Canyon Dam (similar): near-failure from spillway cavitation, so vents installed





•Spillways – Kurobe Dam, Toyama



Hydrology: Stormwater Detention Basins



Hydrology: Stormwater Detention Basins



Stage – storage curve for San Clemente Reservoir in 2011

- Channel conveyance
- •Structures
 - -Weirs
 - -Culverts
 - -Bridges
 - -Stormwater detention basins
- •Flooding
- •Scour
- •Bank armor

•Channel conveyance – Tama River during low flow



- •Channel conveyance Tama River flood
- •How high should we build the levees?
- •How wide should we make the floodplain?



1974





2007

•Bridges – does the bridge have enough clearance not to be submerged in a flood? Or is it designed to be submerged?



Source: The Boston Globe

River Hydraulics •Flooding – modeling a river with HEC-RAS



•Flooding – dam freeboard and bridge clearance



•Flooding – levee height, bridge clearance, floodplain width, residential development



Lagend EG 130 yr Vici 130 yr Ground Bark Sta

•Bridges - scour



2007 神奈川県松田市酒匂川

•Bridges - scour



Source: USACE HEC

•Bank armor – riprap and gabions









Coastal Hydraulics

•Coastal disasters – models determine design water elevation for levees and gates

- -Storm surge
- –Tsunami

 Tidal in-stream power generation – models determine flow speeds and potential for power
Coastal Hydraulics •Storm surge barrier – Rotterdam, the Netherlands



Coastal Hydraulics •Storm surge barrier – London. UK



•Storm surge barrier – Tennozu Isle Gate



Coastal Hydrodynamics

•Storm surge





Storm surge barriers – New Orleans, USA

2

SEABROOK FLOODGATE COMPLEX

Industrial Fas

Vinth Ward

Gulf Intracoast Waterway

Cana

1

GATES AT

OUTFALL

New

Orleans







Pump

5 Miles

1 GATES AT OUTFALL CANALS

Temporary steel gates were built at the entrance to the three outfall canals that drain into Lake Pontchartrain, two of which were breached in Hurricane Katrina. Pumps push rainwater around the gates, which prevent surge from going into the city. The gates will be replaced with more robust permanent structures in 2014.

Source: Army Corps of Engineers

2 SEABROOK FLOODGATE COMPLEX

Mississippi River

LOUISIANA

The complex is part of a larger system of levees, breakwaters and floodwalls to prevent storm surge from entering the Industrial Canal, a large shipping canal near the eastern side of the city that connects Lake Pontchartrain to the Mississippi River. It is intended to work in tandem with the Lake Borgne surge barrier.

Lake Pontchartrain

Foreshore protection to be constructed

JEFFERSON

PARISH

Lake

Cataouatche

Lake

Salvado

3 LAKE BORGNE SURGE BARRIER

An enormous, 1.8-mile floodwall has been built to prevent storm surges from Lake Borgne and the Gulf of Mexico from being funneled into the city. It is designed to protect some of the most vulnerable areas, like New Orleans East, St. Bernard Parish and the Lower Ninth Ward, which were devastated during Hurricane Katrina.

4 WEST CLOSURE COMPLEX

ORLEANS PARISH

ST. BERNARD

4 WEST CLOSURE COMPLEX 3

LAKE BORGNE

SURGE BARRIER

Mercier

Bayou

Lake

Borgne

A \$1 billion system of floodgates. floodwalls, levees and the world's largest pump station will be built a half-mile south of where the Harvey and Algiers Canals meet. The system will prevent storm surge from the Gulf of Mexico from entering west bank communities in the Orleans, Jefferson and Plaquemines Parishes.



Lake Pontchartra

Coastal Hydraulics – Hurricane Katrina levee overtopping









•Tidal in-stream power generation



•Tidal in-stream power generation





- •How do we design how high to make storm surge barriers?
- •How do we decide where to place tidal power generators?
- •Need to calculate storm water levels and tidal current speeds.
- •Use a 2-D shallow water equation model.

•Modeling tusnami with DELFT







Coastal Hydraulics – Tsunami model



-2

-4

-6

-8

Tsunami behavior – model validation









36 min after quake →= arrow size for 5 m/s flow speed



37 min after quake \rightarrow = arrow size for 5 m/s flow speed





39 min after quake \rightarrow = arrow size for 5 m/s flow speed



40 min after quake \rightarrow = arrow size for 5 m/s flow speed



41 min after quake →= arrow size for 5 m/s flow speed



42 min after quake \rightarrow = arrow size for 5 m/s flow speed



43 min after quake →= arrow size for 5 m/s flow speed



44 min after quake $\rightarrow = \text{ arrow size for 5 m/s flow speed}$



45 min after quake →= arrow size for 5 m/s flow speed



46 min after quake →= arrow size for 5 m/s flow speed



47 min after quake \rightarrow = arrow size for 5 m/s flow speed



48 min after quake →= arrow size for 5 m/s flow speed



49 min after quake $\rightarrow = \text{ arrow size for 5 m/s flow speed}$
















36 min after quake



200 400 600 800 1000 1200 1400



38 min after quake



200 400 600 800 1000 1200 1400

39 min after quake



200 400 600 800 1000 1200 1400





42 min after quake



200 400 600 800 1000 1200 1400



44 min after quake



200 400 600 800 1000 1200 1400













- •Coastal levees and shore protection
- •Breakwaters
- •Generating power from waves

Coastal levees and shore protection



Coastal erosion – San Francisco Ocean Beach
Riprap to protect highway and wastewater outfall



- Coastal levees and shore protection
- •Oakland International Airport and San Francisco International Airport
- •How do we design levee height and armor size?
- •Wave modeling







•Hindcast wave field at airports



•Hindcast wave field at airports



- •Hindcast wave field at airports
- •Model results -> Annual maximum wave heights
- •Design wave height





Port of San Francisco Sea Level in 2010



Port of San Francisco Sea Level in 2050 (40 cm higher than today)



Port of San Francisco Sea Level in 2050 (140 cm higher than today)



Modeling breakwaters with SWAN



•Wave power absorbers





Pelamis

Wave Dragon

•Wave power absorbers



BioWave



Aquamarine Power

•Wave power absorbers – wave field





•Wave power absorbers – coastal erosion



Course Outline

- •Hydrology
- •River hydraulics
- •Coastal hydraulics
- •Wave mechanics

Next Week!

•Form groups

- •Each group: bring a laptop
- •Hydrology gauged basins
 - •Statistics theory: normal and log-normal distributed data
 - •Practice: use HEC-SSP to calculate the design (100yr) flood

Ungauged basins

•Theory: rational method

Practice: apply rational method to calculate design flood